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CONDITION SURVEY, WURTSMITH AIR FORCE BASE, MICHIGAN.(U)  
APR 73 P J VEDROS, H T THORNTON

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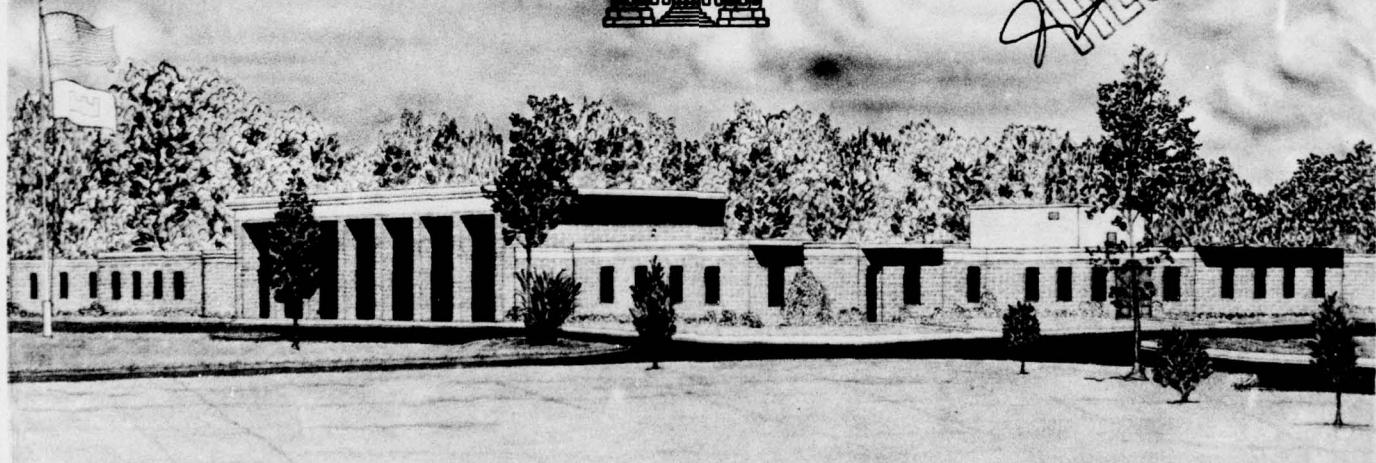
MISCELLANEOUS PAPER S-73-13

## CONDITION SURVEY, WURTSMITH AIR FORCE BASE, MICHIGAN

by

P. J. Vedros, H. T. Thornton, Jr.

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April 1973

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Conducted by U. S. Army Engineer Waterways Experiment Station  
Soils and Pavements Laboratory  
Vicksburg, Mississippi

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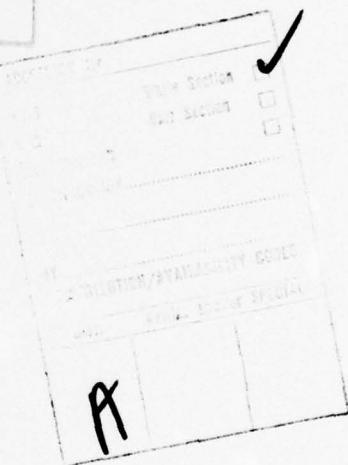
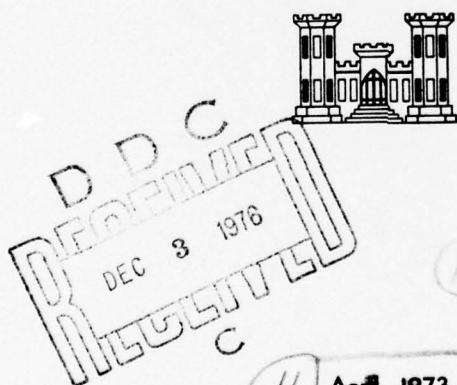


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Foreword

The study reported herein was conducted under the general supervision of the Engineering Design Criteria Branch, Soils and Pavements Laboratory, of the U. S. Army Engineer Waterways Experiment Station (WES), Vicksburg, Mississippi. Personnel involved in the condition survey were Messrs. H. T. Thornton, Jr., Concrete Laboratory, and S. J. Alford and R. N. Gordon, Soils and Pavements Laboratory. This report was prepared by Mr. Thornton and Mr. P. J. Vedros under the general supervision of Messrs. J. P. Sale, R. G. Ahlvin, and R. L. Hutchinson of the Soils and Pavements Laboratory. Appendix A was obtained from the Air Force.

COL Ernest D. Peixotto, CE, was Director of the WES during the conduct of the study and preparation of the report. Mr. F. R. Brown was Technical Director.

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Conversion Factors, British to Metric Units of Measurement

British units of measurement used in this report can be converted to metric units as follows:

<u>Multiply</u>	<u>By</u>	<u>To Obtain</u>
inches	2.54	centimeters
feet	0.3048	meters
miles (U. S. statute)	1.609344	kilometers
square inches	6.4516	square centimeters
square feet	0.092903	square meters
square yards	0.8361274	square meters
pounds (mass)	0.45359237	kilograms
pounds (force) per square inch	0.6894757	newtons per square centimeter
Fahrenheit degrees	*	Celsius or Kelvin degrees

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\* To obtain Celsius (C) temperature readings from Fahrenheit (F) readings, use the following formula:  $C = (5/9)(F - 32)$ . To obtain Kelvin (K) readings, use:  $K = (5/9)(F - 32) + 273.15$ .

CONDITION SURVEY, WURTSMITH AIR  
FORCE BASE, MICHIGAN

Authority

1. Authority for conducting condition surveys at selected airfields is contained in amendment to FY 1972 RDTE Funding Authorization (MFS-MC-5, 16 February 1972), subject: "Air Force Airfield Pavement Research Program," from the Office, Chief of Engineers, U. S. Army, Directorate of Military Construction, dated 18 February 1972.

Purpose and Scope

2. The purpose of this report is to present the results of a condition survey performed at Wurtsmith Air Force Base (WAFB), Michigan, during 1-5 May 1972. The following three major areas of interest were considered in this condition survey:

- 1. The structural condition of the primary airfield pavements.
- 2. The condition of pavement repairs and the types of maintenance materials that have been used at this airfield.
- 3. Any detrimental effects of frost to the pavement facilities.

3. This report is limited to a presentation of visual observations of the pavement conditions, discussion of these observations, and pertinent remarks with regard to the performance of the pavements. No physical tests of the pavements, foundations, or patching materials were performed during this survey. Appendix A to this report presents the annual pavement maintenance plan for WAFB.

Pertinent Background Data

Location

4. WAFB is located in Oscoda County, Michigan, about 1-1/2 miles\*

\* A table of factors for converting British units of measurement to metric units is presented on page vii.

west of the shoreline of Lake Huron and 2-1/2 miles northwest of Oscoda, Michigan. A vicinity map is shown in plate 1.

Topography and soils

5. The airfield is located in a nearly level sand plain. The average elevation within the airfield site is approximately 620 ft above mean sea level. The predominant soil type, "grayling" sand, is a loose, uniformly graded sand ranging from yellowish brown to grayish yellow in color. The soil extends to considerable depths throughout the airfield site. The subgrade and foundation subsoils are composed generally of nonplastic, nonfrost-susceptible uniformly graded fine sand, which is classified as an SP or SP/SM material according to the Unified Soil Classification System.\*

Drainage and water table

6. The surface soil and subsoil are granular and free draining to a known depth of about 20 ft. The groundwater table varies from 15 to 20 ft below the ground surface. Prior to 1955, no subdrains or collection systems for airfield surface runoff had been constructed due to the highly pervious characteristics of the subsoil. Subsequent construction has provided ponding areas and catch basins to expedite runoff from apron areas for relief from standing water during freeze-thaw periods.

Climatic conditions

7. The average annual rainfall in the area is approximately 28 in., with an average annual snowfall of 56.8 in. The mean annual temperature is approximately 45 F. The winters are long and rigorous, while the summers are comparatively short and mild. Climatic data for the period August 1969 to July 1970 are presented in table 1. Rainfall is generally well distributed throughout the year. The rainfall occurs generally as moderate and prolonged rain or as frequent showers, but rarely as a heavy downpour. The snowfall is heaviest from November to

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\* U. S. Department of Defense, "Unified Soil Classification System for Roads, Airfields, Embankments, and Foundations," Military Standard MIL-STD-619B, June 1968, U. S. Government Printing Office, Washington, D. C.

March, but light snowfalls and flurries may occur as late as May and as early as October. Snow remains on the ground throughout the winter. Prevailing winds are from the northwest varying from strong to light. The mean freezing index for the 10-year period 1943-1953 was 950 degree-days. The design freezing index (based on the coldest year in 10) for this same period was approximately 1190 degree-days.

General description of airfield

8. In May 1972, the airfield consisted of a NE-SW (06-24) runway, a parallel taxiway with four connecting taxiways, a SAC operational apron with a connecting taxiway to seven hangar aprons and taxiways, four ADC aprons of various sizes with connecting taxiways to the runway and to other hangar aprons, a warm-up apron at the NE end, and a SAC alert apron with parking stubs and a connecting taxiway. The runway was 300 ft wide and 11,800 ft long. The SAC operational apron was approximately 2200 ft long and 775 ft wide. Taxiways A-D, F, I, and J were 75 ft wide; taxiway E was 100 ft wide; taxiways G and K (the old E-W and NE-SW runways, respectively) were 150 ft wide; and taxiway H was 50 ft wide. The ADC operational apron was approximately 1500 by 575 ft. Other aprons were of various sizes. A layout of the airfield is shown in plate 1.

Previous reports

9. Previous reports concerning WAFB are listed below. Pertinent data were extracted from them for use in this condition survey.

a. Condition survey reports. The following three condition survey reports were prepared by the U. S. Army Engineer Division, Ohio River, CE:

- (1) "Condition Survey Report, Wurtsmith Air Force Base, Michigan," January 1959, Cincinnati, Ohio.
- (2) "Condition Survey Report, Wurtsmith Air Force Base, Michigan," September 1966, Cincinnati, Ohio.
- (3) "Investigation of Pavement Spalling at Wurtsmith Air Force Base, Michigan," December 1966, Cincinnati, Ohio.

b. Pavement evaluation reports. The following four pavement evaluation reports were prepared by the U. S. Army Engineer District, Detroit, CE:

- (1) "Final Report, Airfield Pavement Evaluation, Oscoda Army Airfield, Oscoda, Michigan," 1944.
- (2) "Evaluation of Rigid Pavement, Extension of Northeast-Southwest Runway, Wurtsmith AFB, Michigan," 1954.
- (3) "Evaluation of Rigid Pavement, Apron and Alert Taxiway Extensions, Wurtsmith AFB, Michigan," January 1957.
- (4) "Airfield Evaluation Report, Wurtsmith AFB, Michigan," March 1960.

#### History of Airfield Pavements

##### Construction history

10. The airfield was first developed in 1936 as Camp Skeel, a landing auxiliary to Selfridge Field. The landing surface at that time was treated gravel. In February 1942, two diagonal runways were constructed and surfaced with soil cement. Later in 1942, three runways, connecting taxiways, and an apron were constructed of portland cement concrete (PCC). The field was known during World War II as Oscoda Army Airfield. A new 3000-ft runway extension, a parallel taxiway, and an alert apron and taxiway were constructed in 1951. Additional pavements, consisting generally of various access taxiways, apron areas, and extensions to existing aprons, were constructed during 1955-1957. During 1958-1959, the airfield was rebuilt to accommodate heavy aircraft. Construction completed at this time consisted of a runway, a parallel taxiway, warm-up aprons at each runway end, a hangar apron, alert facilities, and a parking apron of about 188,000 sq yd. Details of the design and construction history of the airfield pavements are presented in table 2. Pavement thicknesses, descriptions, and other details are presented in table 3.

##### Traffic history

11. A detailed traffic record was not available for this study; however, some traffic information was available from previous condition surveys and pavement evaluations. In addition, a traffic count for the year 1971 was obtained during this survey. The B-52 aircraft started

operations at WAFB in July 1960. Prior to this time, the predominant aircraft operating from the airfield were fighters (F-89 series), with occasional operations by aircraft of the C-54 and C-124 classes. The 1966 condition survey indicated that operations at that time consisted of about 250 cycles per month, of which approximately 75 cycles were B-52 aircraft operations, 90 cycles were KC-135 operations, and the remainder were F-101 and transient aircraft operations. (A cycle of aircraft traffic is a combination of one landing and one takeoff.) In addition to the aircraft traffic described above, about three alert taxiing exercises involving 11 to 16 aircraft each were conducted each month. In 1971, there were approximately 11,460 cycles of traffic, of which 1056 cycles were B-52 operations (about 88 per month), 924 cycles were KC-135 operations (about 77 per month), and the remaining cycles were various transient aircraft operations. On normal missions, the average gross weight at takeoff of the B-52 aircraft is approximately 400,000 lb. During alert conditions, the weights are up to about 490,000 lb. It is estimated that approximately 75 percent of the takeoffs are from the northeast (24) end of the runway.

#### Conditions of Pavement Surfaces

##### Pavement inspection

12. The following procedure was used in conducting the pavement inspection of the rigid pavements. Representative features were selected for detailed inspection. The features were then inspected slab by slab, and the defects were recorded. (A slab is the smallest unit, containing no joints, of a pavement feature.) The locations of the individual pavement features, the inspection starting points, and the directions in which the pavements were inspected are shown in plate 1. The results of the rigid pavement survey for those features that were inspected in detail are presented in table 4. This table shows a quantitative breakdown of the various types of defects and a condition rating for each pavement feature inspected in detail. The condition ratings are based primarily on the total percentage of slabs containing no major defects. Although minor defects (spalls, shrinkage cracks,

pop-outs, etc.) are usually discounted in determining the condition ratings, some consideration must be given to them with respect to their effect on the normal operations of aircraft. Consideration must also be given to the minor defects when they may possibly develop into major defects under continuing traffic. The procedures used for determining the condition rating of a pavement are given in Appendix III of Department of the Army Technical Manual TM 5-827-3, "Rigid Airfield Pavement Evaluation," dated September 1965.

#### Runway

13. A detailed inspection (slab by slab) of the runway pavement indicated that it was in excellent structural condition. Eight slabs located in the 15-in.-thick pavement in the outside lane of the interior of the runway (feature R5D) contained the only major defects. However, as is shown in table 4, there were a considerable number of spalls and pop-outs, and some "D" cracking had occurred in the pavement surface. Every slab in the first 1000 ft of the southwest (06) end and almost every slab in the runway interior contained pop-outs (photos 1 and 2). An investigation of the spalling condition on the runway at WAFB was performed in 1966, and the results were published in the report referenced in paragraph 9a. There has been a progression in the number of spalls on the longitudinal joints since the 1966 investigation. The spalls and pop-outs have been repaired with patches of a latex material or an epoxy concrete. Since the patching on the runway has to be accomplished between aircraft operations, a fast setting epoxy patch is used. In some cases a poor patching job has resulted because of the rush. The repair of spalls and pop-outs is a costly, major maintenance problem at this airfield.

14. Structurally, the pavements are performing satisfactorily under traffic of the B-52 aircraft now using the pavements. Five B-52 pilots and twenty-two KC-135 pilots were asked to rate the riding quality of the runway. Sixty-three percent rated it as smooth, while thirty-seven percent rated it as fair.

#### Primary taxiways

15. The primary taxiway system consists of taxiways A, B, and E;

the SAC operational apron taxiway; and apron access taxiways 1 and 2. All of these primary taxiways consist of 21-in.-thick PCC, and all were in excellent structural condition, with only three major defects observed in the entire system. Pop-outs and spalls were the predominant minor defects in the system. Thirty-six slabs of taxiway A contained what was identified as "D" cracking, a condition which can become severe enough to develop into spalls. Spalls of 1 to 4 ft in length are occurring along longitudinal joints at the rate of 5 to 7 per year in the southwestern two-thirds of taxiway A.

Aprons and miscellaneous pavements

16. The 17-in.-thick pavement of the SAC operational apron was in excellent structural condition at the time of inspection. Approximately 20 percent of the apron area contained pop-outs and spalls.

17. The 20-in.-thick pavement of the northeast warm-up apron was in excellent structural condition. Approximately 15 percent of this apron contained pop-outs and spalls.

18. The SAC alert facility, which consists of nine parking stubs and taxiway F (20-in.-thick pavement), was in excellent structural condition. However, 100 percent of the slabs in the parking stubs and 87 percent of the slabs in the taxiway contained pop-outs. There were numerous pop-out patches in the taxiway; and, in the 600 ft closest to the runway, some deterioration of the patches was observed.

19. Connecting taxiways C and D were in excellent condition, with only a minor number of pop-outs and joint spalls. The 10-in.-thick pavement of taxiway J (feature T6B) was not surveyed in detail, but it was noted that structural distress had occurred in the taxiway up to taxiway K due to parking of heavy aircraft during alert exercises.

20. The remaining facilities were not selected for detailed inspection; however, a cursory inspection was made of the pavements. It was noted that taxiway K (the original NE-SW runway) was in excellent to poor condition. In 1970, a section of this taxiway (adjacent to taxiway J), which had an original pavement of 6 in. of PCC over 6 in. of soil cement, was replaced with 14 in. of PCC so that taxiway K could be used as a turnaround apron for unloading cargo. In 1971, a section of

taxiway K (adjacent to taxiway G), which is used as a power check pad, was replaced with 9 in. of PCC, and a 10- by 10- by 3-ft reinforced concrete tiedown anchor was constructed. The new pavements were in excellent condition, and the original pavements, though not in use, were in poor condition (photos 3 and 4, respectively). The ADC operational apron was considered to be in good condition, even though a number of the slabs had major defects. In 1969, about 4500 sq yd of this apron area (originally 7 in. of PCC) was replaced with 9 in. of PCC (photo 5). Taxiway G (the old E-W runway), which received an asphaltic-concrete overlay on its center in 1967, was in fair condition. In the 1972 inspection, these pavements (taxiways G, J, and K) showed signs of cracking from overloads by fighters and transient aircraft.

#### Maintenance

21. Maintenance of the older pavements has consisted mostly of replacing failed concrete slabs and sealing joints and cracks. Repair of pop-outs and joint spalling seems to be the major maintenance problem in the newer pavements (those constructed since 1958). Records show that during 1962-1963, when the first major program to repair pop-outs was initiated, approximately \$475,000 was spent on overall maintenance of airfield pavements. About \$260,000 of this amount was spent on pop-out repairs. Maintenance expenditures after this time are as follows:

<u>Date</u>	<u>Expenditure</u>	<u>Description of Repair</u>
October 1965	\$30,873	Place slurry seal on shoulders and overruns
September 1967	\$16,880	Perform general pop-out and pavement repairs. Seal joints for 600 ft at each end of runway, on SAC operational apron, and on ADC operational apron
October 1967	\$58,000	Seal joints and repair pop-outs on runway, primary taxiways, and SAC operational apron and alert facility

(Continued)

<u>Date</u>	<u>Expenditure</u>	<u>Description of Repairs</u>
August 1968	\$33,780	Repair 60,000 pop-outs without coring (40,000 with latex, 20,000 with epoxy)
	\$32,350	Repair 33,000 pop-outs (18,000 with latex, 15,000 with epoxy)
October 1969	\$97,380	Replace 210 10- by 20-ft by 9-in. con- crete slabs in ADC operational apron
	\$73,000	Replace some old concrete with 9-in. PCC; repair a 480-sq-ft spalled area and 1300 pop-outs; and seal 6000 ft of joints on taxiway G
October 1970	\$158,220	Replace old concrete with 9-in. PCC on taxiway G
Summer 1971	--	Place double bituminous seal on over- runs
Since 1967	\$140,000	Repair pop-outs by shops on in-house basis at a cost of approximately \$0.60 per hole
FY 1973		Plan to repair 20,000 new and recap 30,000 old pop-outs

#### Condition of Joint Seal Materials

22. The last major joint seal repair was accomplished in 1967. At this time, all the joints in the runway, the primary taxiways, the SAC alert facility, the SAC operational apron, and the ADC operational apron were resealed. At the time of the 1972 survey, the joint seal material in these areas was in generally good condition. Joint seal material in other areas was in fair to poor condition.

#### Evaluation

23. The latest pavement evaluation report for WAFB was prepared in 1960 (see paragraph 9b). Since some changes in gear configurations and methods of evaluation have been made since that time, a new evaluation table has been prepared (table 5). The physical properties of the materials as determined in previous evaluations were used for

determining the load-carrying capabilities of the pavements. An evaluation of the effects of frost action was not required for most of the pavements because of the nonfrost-susceptible nature of the sandy sub-grade; however, an evaluation of the frost capacities of taxiway H and the AIC operational apron was made because of the presence of frost-susceptible base courses under these pavement facilities.

#### Conclusions

24. The following remarks summarize the findings of the 1972 inspection:

- a. The heavy-load pavements are in excellent condition, with the predominant defects being joint spalls and pop-outs.
- b. Some "D" cracking was observed in the outer lanes of the runway and in taxiway A. Spalls will probably develop in these areas in the future.
- c. The longitudinal joint failures in the southwestern two-thirds of taxiway A could become a serious maintenance problem in the future.
- d. Joint seal materials in primary traffic areas are in generally good condition.
- e. Pop-outs are a major maintenance problem in primary traffic areas, and they require continuous repair.
- f. Spall repairs made with epoxy concrete and PCC are performing satisfactorily.
- g. Pop-out repairs made with epoxy concrete and latex are also performing satisfactorily.
- h. Most of the original pavement constructed in 1942 is in poor condition.

Table 1  
Climatic Data,\* August 1969-July 1970

Month	Average Daily Temperature, F			Precipitation, in.	
	Max	Min	Mean	Rainfall	Snowfall
January	27	12	20	1.5	14.0
February	30	13	22	1.7	14.0
March	37	21	29	1.9	10.3
April	51	33	42	2.8	2.7
May	63	42	52	3.0	0.1
June	73	52	63	2.7	Trace
July	78	57	67	3.0	Trace
August	77	56	67	2.7	None
September	68	49	59	2.8	Trace
October	58	40	49	2.2	0.1
November	44	30	37	2.5	4.8
December	31	18	25	1.6	10.8
Annual	53	35	45	28.4	56.8

\* Furnished by WAFB.

Table 2  
Airfield Construction History

Feature No.	Pavement Facility	Pavement		Construction Year(s)	Agency	Design Criteria
		Thickness in.	Type			
T14B	Taxiway K (original NE-SW runway)	6	PCC	1942	CE*	Gross load - 74,000 lb Wheel load - 37,000 lb Tire pressure - 67-1/2 psi
T8B	Taxiway G (original E-W runway)	10-7-10	PCC	1942	CE	--
A6B	ADC operational apron	10-7-10	PCC	1942	CE	--
A12B	ADC alert apron and taxiway	10	PCC	1951	CE	Engineering Manual (EM) for Military Construction Wheel load - 25,000 lb
A10B	ADC hangar apron and taxiway	10	PCC	1951	CE	Tire pressure - 200 psi
A11B, A13B	ADC alert taxiway and apron additions	10	PCC	1955	CE	
A9B	ADC parking aprons	10	PCC	1955	CE	
A7B	Hangar access apron	14	PCC	1955	CE	Gear load - 80,000 lb Dual wheels - 37 in. c-to-c Tire pressure - 150 psi
A8B	Hangar apron and taxiway	14	PCC	1955	CE	Gear load - 80,000 lb Dual wheels - 37 in. c-to-c Tire pressure - 150 psi
A14B	Rear apron	3	AC	1956	CE	--
T6B	Taxiways J and I	10	PCC	1959	CE	Wheel load - 25,000 lb 100-sq-in. contact area
T7B	Taxiway G (west 700 ft)	10	PCC	1959	CE	Wheel load - 25,000 lb 100-sq-in. contact area
A3B	Hangar aprons and taxiways	14	PCC	1958-59	CE	Gear load - 160,000 lb
A1B	SAC operational apron	17	PCC	1958-59	CE	EM 1110-45-303 Gear load - 265,000 lb
T5A	SAC operational apron taxiway	21 and 17	PCC	1958-59	CE	37- by 62- by 37-in. spacing 267-sq-in. contact area
T4A, T12A	SAC operational apron access taxiways	21	PCC	1958-59	CE	
T1A, T3A	Taxiways B and E	21	PCC	1958-59	CE	
T2A	Taxiway A	21 and 20	PCC	1958-59	CE	
T10C, T11C	Taxiways C and D	17	PCC	1958-59	CE	
R1A, R7A	Runway ends (200-ft-wide section, 500 ft each end)	21	PCC	1958-59	CE	
R3B, R6B	Runway (100-ft-wide center section, 2d 500 ft each end)	20	PCC	1958-59	CE	
R4C	Runway interior (100-ft-wide center section)	17	PCC	1958-59	CE	
R5D	Runway interior (100-ft each side)	15	PCC	1958-59	CE	
A5B	NE warm-up apron	20	PCC	1958-59	CE	
T13B, A2B	SAC alert aprons and taxiway F	20	PCC	1958-59	CE	
T8B	Taxiway G (original E-W runway) 2800 ft by 75 ft	2.5	AC overlay	1967	CE	
T14B	Taxiway K (original NE-SW runway) Approximately 500 ft of end replaced	14	PCC	1970	Contract	--
T14B	Taxiway K (power check pad). Approximately 700- by 50-ft area replaced	9	PCC	1971	Contract	--

\* CE denotes Corps of Engineers.

Table 3  
SUMMARY OF PHYSICAL PROPERTY DATA

Warranted, MPa, MPa	FACILITY	OVERLAY PAVEMENT				PAVEMENT			BASE		SUBGRADE		GENERAL CONDITION OF AREA CONSIDERED	
		FACILITY NUMBER AND IDENTIFICATION	LENGTH FT	THICK. IN.	DESCRIPTION	FLEX. STR. PSI	THICK. IN.	DESCRIPTION	FLEX. STR. PSI	THICK. IN.	CLASSIFICATION	CBR K	CLASSIFICATION	
SLA	SLA 10-10 roadway; 1st 500 ft each end	500	200		Portland cement concrete	21				600				Good (EP)
SLA	SLA 10-10 roadway; 1st 500 ft each end, north 10-ft edge	500	100			20	Portland cement concrete, 10-ft paving to 10-ft transition to 20-in.							Good (EP)
SLB	SLB 10-10 roadway; 2nd 500 ft each end	500	100			20	Portland cement concrete, 10-ft paving to 10-ft transition to 20-in.			600				Good (EP)
SLC	SLC 10-10 roadway interior	2,800	100			17	Portland cement concrete			600				Good (EP)
SLD	SLD 10-10 roadway interior on side lanes	11,000	100			15	Portland cement concrete			600				Good (EP)
TIA	TIA Factory B	1,133	75			21	Portland cement concrete			800				Good (EP)
TIA	TIA Factory A	10,462	75			20	Portland cement concrete			800				Good (EP)
TIA	TIA Factory E	850	100			21	Portland cement concrete			800				Good (EP)
TIA	TIA operational apron	250	75			21	Portland cement concrete			810				Good (EP)
TIA	TIA service roadway (2)	600	75			17-1/2-17	Portland cement concrete			810				Good (EP)
TIA	TIA operational apron roadway	5,855	75			17	Portland cement concrete			810				Good (EP)
TIB	TIB roadway F	2,800 <sup>a</sup>	75			10	Portland cement concrete			780				Good (EP)
TIB	TIB roadway G	600 <sup>a</sup>	75			10	Portland cement concrete			800				Good (EP)
TIB	TIB roadway D	1,800 <sup>a</sup>	150	2.5	Asphaltic concrete ( $h_{c,r} = 1.5$ )	10-1/2-15	Portland cement concrete	2 to 6 (average)		600				Good (EP)
TIB	TIB roadway H	1,650	90			9	Portland cement concrete			600	4 to 8 (based upon clay)			Good (EP)
TIC	TIC connecting roadway C and D					17	Portland cement concrete			810				Good (EP)
ADB	ADB operational apron	2,185	775			17	Portland cement concrete			810				Good (EP)
ADB	ADB short static T13B (2) and roadway F					20	Portland cement concrete			800				Good (EP)
ADB	ADB tangent aprons (6) and roadway F					14	Portland cement concrete			810				Good (EP)
TIB	TIB roadway F (10-10 paving)	1,600 <sup>a</sup>	150			14-1/2-19	Portland cement concrete	2 to 6 (average)		600				Good to excellent

Table 3 (continued)

## SUMMARY OF PHYSICAL PROPERTY DATA

Warranted AASHTO Min. FACILITY	FACILITY NUMBER AND IDENTIFICATION	OVERLAY PAVEMENT				PAVEMENT				BASE				SUBGRADE	GENERAL CONDITION OF AREA CONSIDERED	
		LENGTH FT	WIDTH FT	THICK. IN	DESCRIPTION	FLX. STR. PSI	THICK. IN	DESCRIPTION	FLX. STR. PSI	THICK. IN	CLASSIFICATION	CBR ON K	CLASSIFICATION	CBR ON K		
A-35 NS warm-up Apron	Varies						100	Portland cement concrete	840						Good	Excellent
A-36 AOC operational apron	1,500 <sup>2</sup> 572				10 <sup>2</sup> - 10	Portland cement concrete	800	4 to 8 Mixed sand and clay 1 <sup>2</sup> to 2 <sup>2</sup>	790	Sand (10)				Good	Good	
A-37 Blower access open and tacway	Varies				14	Portland cement concrete	850								Good (10)	Excellent
A-38 Maintenance linear apron and tacway	Varies				14	Portland cement concrete	850								Good (10)	Excellent
A-39 AOC parking apron	Varies				10	Portland cement concrete	800								Good (10)	Excellent
A-40 AOC linear apron and tacway	Varies				10	Portland cement concrete	840								Good (10)	Excellent
A-41 AOC alert apron and tacway	Varies				10	Portland cement concrete	850								Good (10)	Excellent
A-42 AOC alert apron and tacway	Varies				10	Portland cement concrete	840								Good (10)	Excellent
A-43 AOC alert apron extension	Varies				10	Portland cement concrete	800								Good (10)	Excellent
A-44 Outer access apron	100 107				3	Impastetic granular	6	0.000131461 0.02790014 90	790	Sand (10)				Good	Good	

Table 4

## SUMMARY OF DATA - RIGID PAVEMENT CONDITION SURVEY

DATE: May 1972

AIRFIELD: ATLANTA, GAMATERIAL: ASPBWEIGHT: 100

## NO. OF SLABS CONTAINING INDICATED DEFECTS

FEATURE NO.	DESIGNATION	SLAB SIZE, FT	APPROX NO. OF SLABS	PAVE. THICK. IN.	NO. OF SLABS CONTAINING INDICATED DEFECTS												% OF SLABS NO. DEFECTS	% OF SLABS NO. DEFECTS		
					I	—	＼	△	*	K	~~	S	J	Ψ	J	Φ	M			
R1A	NE-SW runway 24 end 1st	25 by 25	240	20 and 21						4		7	11	18			27	8	82	
R2B	1st 500 ft									5		13	26	43			84	9	28	
R3B	NE-SW runway 24 end 2nd	25 by 25	240	15 and 20															100	
R5D	2nd 500 ft																		Excellent	
R4C	NE-SW runway interior	25 by 25	1704	15 and 17	7	1				4		50	188	260			4563	12	99	
R5D	NE-SW runway 6 end 2nd 500 ft	25 by 25	240	15 and 20								2	3	14					Excellent	
R6B																		0	100	
R7A	NE-SW runway 6 end 1st 500 ft	25 by 25	240	15 and 20															Excellent	
R5D																		0	100	
TLA	Taxiway B	25 by 25	184	21						1		2							Excellent	
T2A	Taxiway A	25 by 25	1398	20-21- 20						1		35	27	57			190	36	88	
T3A	Taxiway E	25 by 25	146	21								2	4	3	10			125	12	100
	SAC Operational																		Excellent	
T4A	apron access taxiway 1	25 by 25	52	21	1							1	3	1	2			44	6	99
T12A	SAC operational apron access taxiway 2	25 by 25	50	21													33	34	100	

REMARKS:

## LEGEND:

- LONGITUDINAL CRACK
- TRANSVERSE CRACK
- ＼ DIAGONAL CRACK
- △ CORNER BREAK
- \* SHATTERED SLAB
- K KEYED JOINT FAILURE
- ~~ SHRINKAGE CRACK
- S SCALING
- J SPALL ON TRANSVERSE JOINT
- J SPALL ON LONGITUDINAL JOINT
- Φ CORNER SPALL
- Φ SETTLEMENT
- M MAP CRACKING
- P PUMPING JOINT
- O POP-OUT
- C UNCONTROLLED CONTRACTION CRACK
- D T-D CRACKING

Table 4 (Continued)

DATE: May 1972

## SUMMARY OF DATA - RIGID PAVEMENT CONDITION SURVEY

SUMMARY OF DATA - RIGID PAVEMENT CONDITION SURVEY														NO. OF SLABS CONTAINING INDICATED DEFECTS				NO. OF SLABS NO. OF MAJOR DEFECTS								
FEATURE No.	DESIGNATION	SLAB SIZE FT.	ADDRESS No. of Slabs	Pav. Thickness in.										I	- -	Δ	* K	~ S	J	Ψ	J	⊕ M	P	O	C	D
				1	2	3	4	5	6	7	8	9														
TSA	SAC operational apron roadway	25 by 25	366	17-21- 17																143	9	58	99			
T13B	Taxiway F	25 by 25	144	20																361	13	109	Excel- lent			
TA1B	SAC operational apron	25 by 25	2241	17																138	6	82	Excel- lent			
TA2B	Alert stats 1-9	25 by 25	675	20																675	0	100	Excel- lent			
TA5B	NE warm-up spron	25 by 25	270	20																22	86	99	Excel- lent			
T10C	Taxiway C	25 by 25	118	17																7	89	100	Excel- lent			
T11C	Taxiway D	25 by 25	120	17																10	6	80	Excel- lent			

10

I LONGITUDINAL CRACK  
 — TRANSVERSE CRACK  
 \ DIAGONAL CRACK  
 ▲ CORNER BREAK  
 △ SHATTERED SLAB  
 \* KEYED JOINT FAILURE

**M** MAP CRACKING  
**P** PUMPING JOINT  
**O** POP-OUT  
**C** UNCONTROLLED CONTRACTION CRACK  
**D** "D" CRACKING

LEGEND:	I	LONGITUDINAL CRACK	~	SHRINKAGE CRACK
	—	TRANSVERSE CRACK	S	SCALING
	\	DIAGONAL CRACK	J	SPALL ON TRANSVERSE JOINT
	△	CORNER BREAK	J	SPALL ON LONGITUDINAL JOINT
	*	SHATTERED SLAB	J	CORNER SPALL
	K	KEYED JOINT FAILURE	Φ	SETTLEMENT

WES FORM NO. 2004  
JUN 1972

Table 5  
SUMMARY OF PAVEMENT EVALUATION

NAME OF AIRFIELD: Wurtsmith AFB, Mich.		LOAD-CARRYING CAPACITY IN LB OF GROSS PLANE LOAD FOR INDICATED LANDING GEAR TYPES AND CONFIGURATIONS											
DATE OF EVALUATION MONTH: May YR: 1972		TRICYCLE ARRANGEMENT											
FEATURE NO.	DESIGNATION	PAVEMENT OPERATIONAL USE	SINGLE TIRE PRESSURE 10 psi	SINGLE TIRE PRESSURE 10 psi	SINGLE TIRE PRESSURE 10 psi	T 28 IN. C-C 246 SQ IN. CONTACT AREA EACH TIRE	SINGLE TANDEM 40 SQ IN. CONTACT AREA EACH TIRE	T 30 IN. C-C 265 SQ IN. CONTACT AREA EACH TIRE	T 34 IN. C-C 265 SQ IN. CONTACT AREA EACH TIRE	T 34 TANDEM 33 IN. x 46 IN. 208 SQ IN. CONTACT AREA EACH TIRE	BICYCLE	TRI TANDEM 265 SQ IN. CONTACT AREA EACH TIRE	REMARKS
R1A	NE-SW runway 1st 500 feet each end	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	610,000+	
R7A	NE-SW runway 1st 500 feet each end north 100 feet edge	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	610,000+	
R3B	NE-SW runway 2nd 500 feet each end	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	610,000+	
R6B		Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	610,000+	
R4C	NE-SW runway	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	610,000+	
R5D	Interior	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	610,000+	
T1A	Taxiway F	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	610,000+	
T2A	Taxiway A	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	610,000+	
T3A	Taxiway E	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	610,000+	
T1A	SAC operational taxway access taxways (2)	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	580,000	
T1A		Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	580,000	
T1A	spur taxiway	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	580,000	
T2B	Taxiways 1 and 3	Capacity	90,000	70,000	130,000	135,000	200,000+	180,000	210,000	320,000	800,000+	(a)	
T7B	Taxiway G	Capacity	95,000	75,000	145,000	145,000	200,000+	170,000	230,000	340,000	800,000+	210,000	
T8B	Taxiway G	Capacity	85,000	65,000	130,000	130,000	200,000+	150,000	205,000	285,000	800,000+	(a)	
T9B	Taxiway H*	Capacity	75,000	60,000	120,000	115,000	185,000	130,000	190,000	285,000	730,000	(a)	
T10C	Taxiways C and D	Forest capacity	95,000	14,000	75,000	75,000	115,000	85,000	(a)	155,000	140,000	(a)	
		Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	330,000+	380,000+	800,000+	610,000+	

Note: + sign denotes allowable gross loading greater than maximum gross weight of any existing aircraft having indicated gear configuration.  
 (a) denotes allowable gross loading less than maximum gross weight of any existing aircraft having indicated gear configuration.  
 \* Evaluation of frost capacity required due to presence of frost-susceptible base course beneath feature.

Table 5 (Continued)  
SUMMARY OF PAVEMENT EVALUATION

NAME OF AIRFIELD: Kurtissmith AFB, Mich.		LOAD-CARRYING CAPACITY IN LB OF GROSS PLANE LOAD FOR INDICATED LANDING GEAR TYPES AND CONFIGURATIONS									
DATE OF EVALUATION MONTH: Nov YR: 1972		TRICYCLE ARRANGEMENT									
FEATURE NO.	DESIGNATION	PAVEMENT OPERATIONAL USE	SINGLE 100-SQ-IN. TIME PRESSURE	SINGLE 100-SQ-IN. CONTACT AREA	TWIN 24-SQ-IN. CONTACT AREA EACH TIRE	SINGLE 60-SQ-IN. CONTACT AREA EACH TIRE	TWIN 36-SQ-IN. CONTACT AREA EACH TIRE	TWIN 44-SQ-IN. CONTACT AREA EACH TIRE	TWIN TANDEM 31-IN. x 46-IN. CONTACT AREA EACH TIRE	BICYCLE	
A1B	SAC operational apron	Capacity	155,000+	85,000+	155,000+	270,000+	200,000+	330,000+	360,000+	800,000+	1,600,000
A2B	SAC alert stabs (19) and taxiway F	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	320,000+	360,000+	800,000+	1,600,000
A3B	SAC hangar aprons (6) and taxiway F	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	320,000+	360,000+	800,000+	1,600,000
T14B	Taxiway K	Capacity	30,000	23,000	55,000	50,000	80,000	60,000	(a)	125,000	360,000 (a) Reduced by 25%
A1B	NE warm-up apron	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	360,000+	800,000+	1,600,000
A2B	ADC operational apron *	Capacity	55,000	40,000	75,000	85,000	130,000	95,000	125,000	205,000	530,000 (a)
A2B	Maintainance apron and taxiway	Front capacity	35,000	30,000	55,000	55,000	80,000	60,000	(a)	115,000	330,000 (a)
A1B	Hanger access apron and taxiway	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	360,000+	800,000+	1,600,000
A2B	Maintainance apron and taxiway	Capacity	155,000+	85,000+	155,000+	220,000+	200,000+	330,000+	360,000+	800,000+	1,600,000
A2B	ADC parking apron	Capacity	90,000	70,000	140,000	130,000	200,000+	135,000	215,000	330,000	800,000+ (a)
A10B	ADC hanger apron and taxiway	Capacity	95,000	75,000	155,000+	145,000	200,000+	165,000	225,000	340,000	800,000+ 235,000
A11B	ADC alert apron and taxiway addition	Capacity	90,000	70,000	140,000	135,000	200,000+	155,000	215,000	330,000	800,000+ (a)
A12B	ADC alert apron and taxiway	Capacity	95,000	75,000	155,000+	145,000	200,000+	165,000	225,000	340,000	800,000+ 235,000
A13B	ADC alert apron extension	Capacity	90,000	70,000	140,000	135,000	200,000+	155,000	215,000	330,000	800,000+ (a)
A14B	Rear access apron	Capacity	60,000	60,000	75,000	110,000	150,000	140,000	190,000	330,000	800,000+ (a)

\* Evaluation of front capacity required due to presence of front-susceptible base course tenth feature.

ME FORM NO. 999

EDITION OF AUG 1960 IS OBSOLETE.

JUNE 1972

(2 of 2 sheets)



Photo 1. Pop-outs in interior of runway

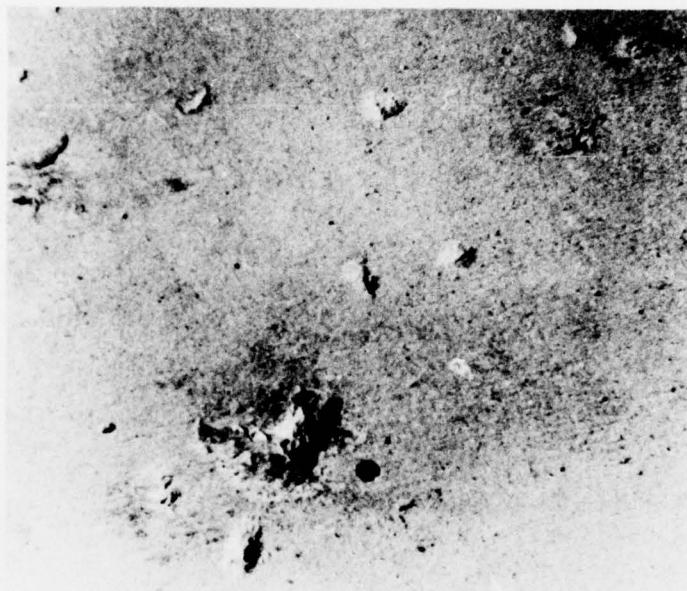


Photo 2. Pop-outs in traffic area D (outside edges of runway)

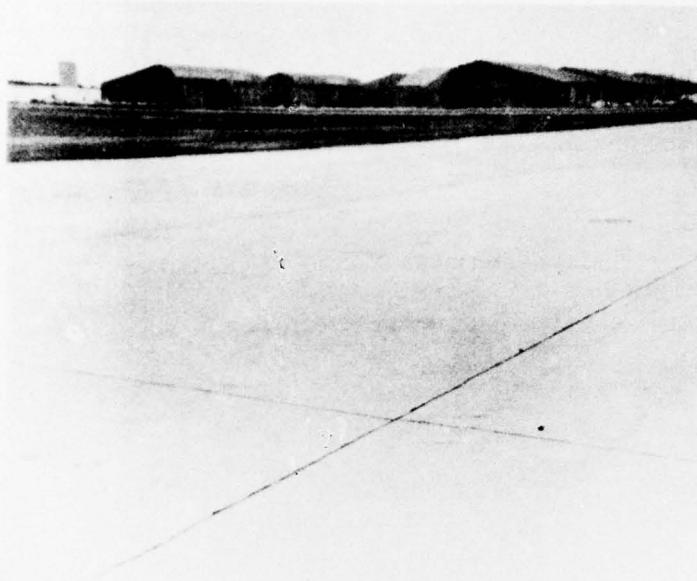


Photo 3. New 14-in. PCC surface on taxiway K

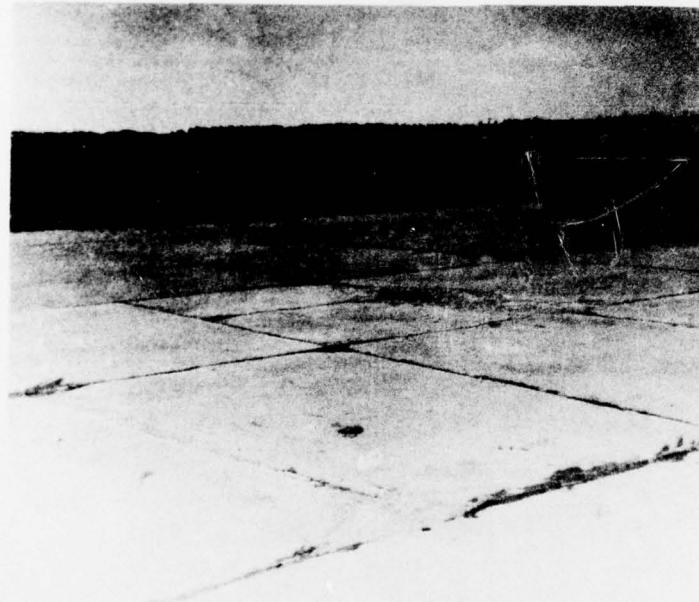


Photo 4. Original pavement on taxiway K;  
condition rated poor

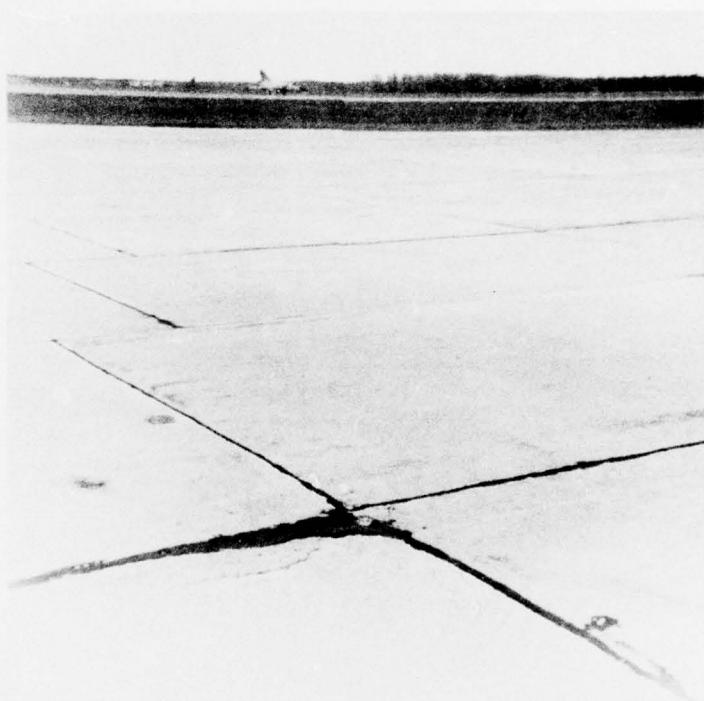
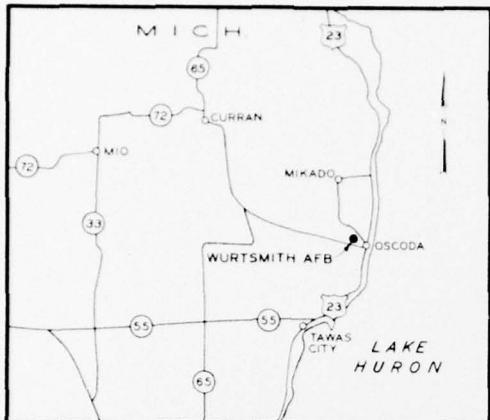


Photo 5. ADC operational apron



VICINITY MAP  
SCALE IN MILES

10 0 10 20

LEGEND

- [Hatched Box] PORTLAND CEMENT CONCRETE (PCC)
- [Dashed Box] ASPHALTIC CONCRETE (AC)
- [Dotted Box] ASPHALTIC CONCRETE OVER PORTLAND CEMENT (AC/PCC)
- [White Box] DOUBLE BITUMINOUS SURFACE TREATMENT (DBST)
- [Solid Black Box] BLAST PAVEMENT (AC-NON TRAFFIC)

RIA 21" PCC ← FEATURE DESIGNATION (SEE NOTE 1)  
21" PCC ← SURFACE THICKNESS AND TYPE

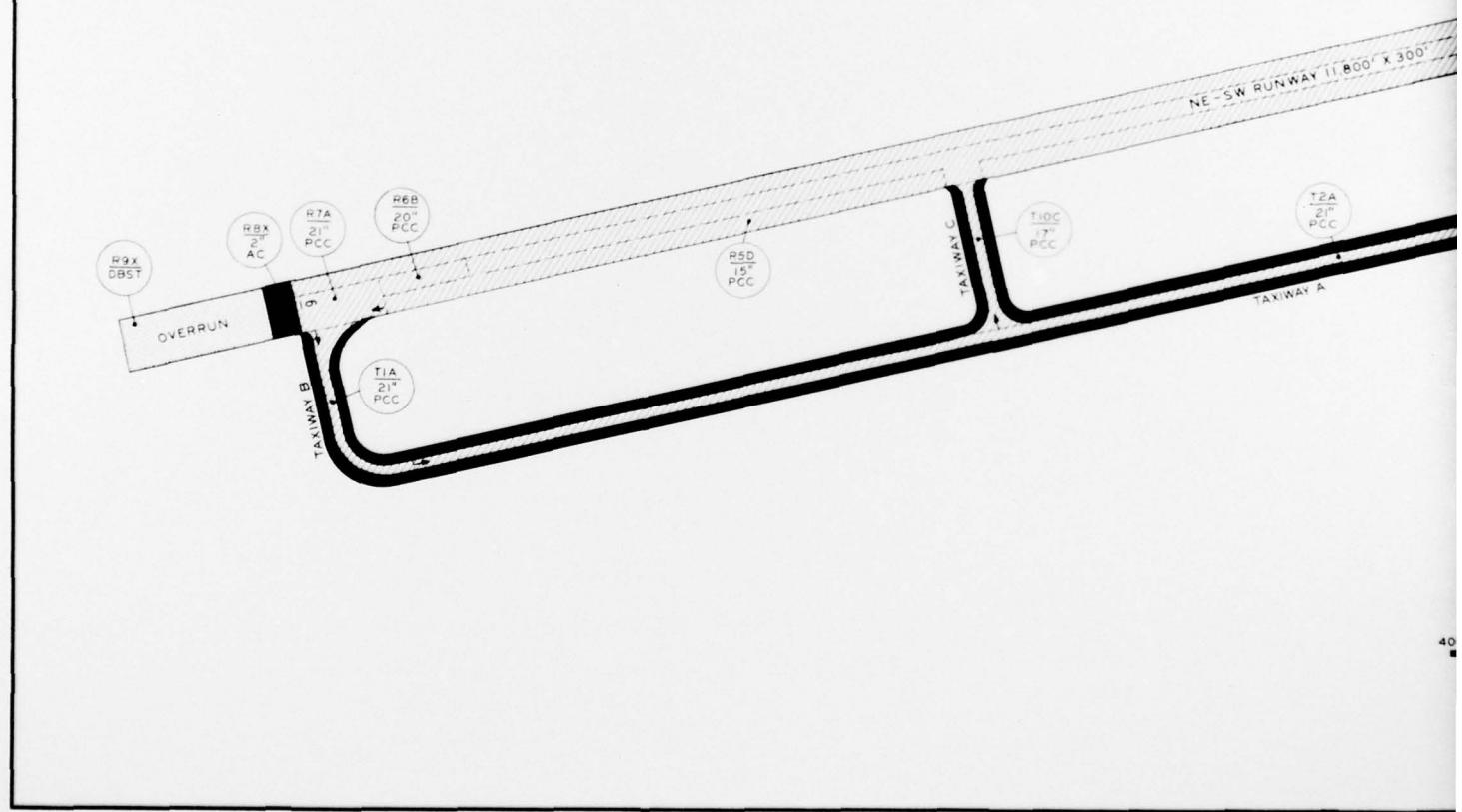
TYPE OF FEATURE

- R - RUNWAY
- T - TAXIWAY
- A - APRON

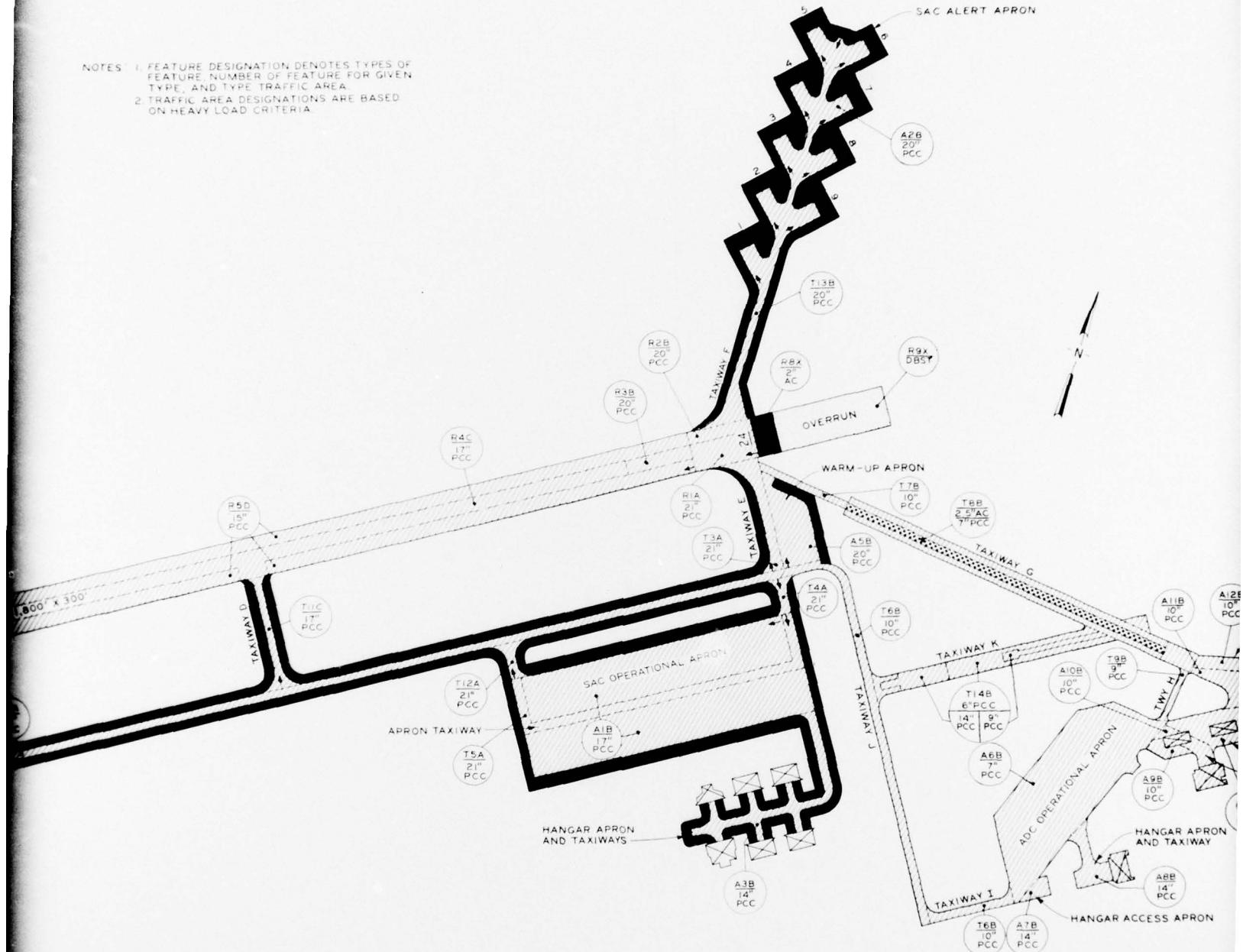
TYPE TRAFFIC AREA (SEE NOTE 2)

- A - A TYPE TRAFFIC
- B - B TYPE TRAFFIC
- C - C TYPE TRAFFIC
- D - D TYPE TRAFFIC
- X - NO TRAFFIC TYPE ASSIGNED

← DIRECTION OF SURVEY



NOTES: 1. FEATURE DESIGNATION DENOTES TYPES OF FEATURE, NUMBER OF FEATURE FOR GIVEN TYPE, AND TYPE TRAFFIC AREA.  
2. TRAFFIC AREA DESIGNATIONS ARE BASED ON HEAVY LOAD CRITERIA.



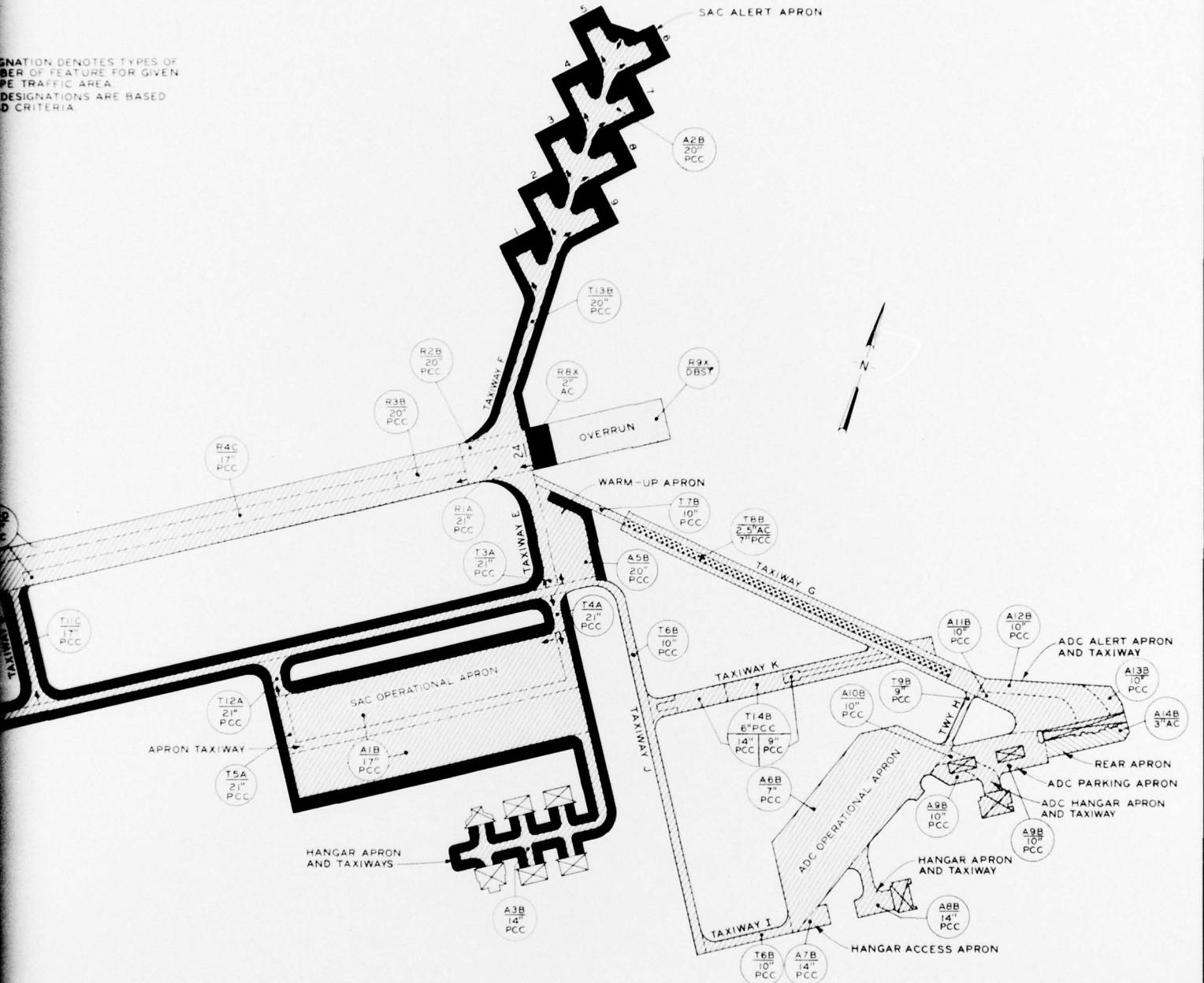
SCALE IN FEET

400 0 400 800 1200

WURTSMITH AFB

## AIRFIELD LAYOUT AND PAVE

**G**NATION DENOTES TYPES OF  
**I**BER OF FEATURE FOR GIVEN  
**P**PE TRAFFIC AREA.  
**D**ESIGNATIONS ARE BASED  
**D**CRITERIA.



**WURTSMITH AFB**

## AIRFIELD LAYOUT AND PAVEMENT PLAN

PLATE 1

3

APPENDIX A: WAFB ANNUAL PAVEMENT MAINTENANCE PLAN

Description	Pavement Type	Year(s) Constructed	Maintenance Completed	Present and Proposed Maintenance
Primary runway (11,800 by 300 ft) 393,333 sq yd of 15-, 17-, 20-, and 21-in.-thick pavement	Rigid heavy	1968-59	FY 70-71: 40,000 pop-outs and 30,000 spalls repaired in house	FY 71: repair 20,000 pop-outs and re- cap 30,000 old pop-outs Project WUR 92-8: replace 4 slabs; de- sign completed, but no funds available
Two overruns (150 by 300 ft) 10,000 sq yd of 2-in.-thick blast pavement	Flexible light	1959	FY 66: slurry seal applied FY 70: random cracks sealed in house	Project WUR 53-0: apply patches and double bituminous seal (under con- tract)
Two overruns (850 by 300 ft) 56,667 sq yd of double bituminous surface treatment	Double bituminous seal over gravel base	1959	FY 66: slurry seal applied FY 70: random cracks sealed in house	Project WUR 53-0: apply patches and double bituminous seal (under contract)
SAC alert taxiway, 34,304 sq yd of 20-in.-thick pavement	Rigid heavy	1968-59	FY 70-71: spalls repaired in house	Yearly in-house maintenance program
ADC alert taxiway: 8,183 sq yd of 10-in.-thick pavement; 21,666 sq yd of 10-7-10-in. pavement	Rigid light; flexible overlay	1942 (ex- tended during 1954)	FY 68: bituminous overlay ap- plied to pavement constructed in 1942 FY 70-71: random cracks sealed in house	Yearly in-house maintenance program
SAC operational apron access taxiways 14,120 sq yd of 20-21-20-in. pavement	Rigid heavy	1968-59	FY 70-71: spalls repaired and random cracks sealed in house	Yearly in-house maintenance program
ADC operational apron access taxiway 26,125 sq yd of 10-in. pavement	Rigid light	1968-59	FY 70: spalls repaired and ran- dom cracks sealed in house	Yearly in-house maintenance program with limitations according to avail- able man-hours
ADC operational apron access taxiway 14,630 sq yd of 10-7-10-in. pavement	Rigid light	1942	FY 70: damaged slabs replaced under contract	Yearly in-house maintenance program with limitations according to avail- able man-hours
Primary runway connecting taxiways 124,575 sq yd of 17-in. and 20- 21-20-in. pavement	Rigid heavy	1968-59	FY 70: spalls repaired and bad joints sealed in house	FY 71: repair pop-outs and spalls in house
ADC access taxiway, 2,361 sq yd of 10-7-10-in. pavement	Rigid light	1942	FY 67: joints sealed FY 70: spalls repaired in house	Yearly in-house maintenance program
SAC hangar apron and taxiways 16,429 sq yd of 14-in.-thick pavement	Rigid light	1959	FY 67: joints sealed and dam- aged slabs replaced FY 68-70: spalls repaired in house	FY 71: Project WUR 101-0: replace 15 damaged slabs (completed in Oct 71)
ADC hangar access apron 35,509 sq yd of 10- and 14-in.- thick pavement	Rigid light	1942 1951 1967	FY 70: spalls repaired and joints sealed in house	Yearly in-house maintenance program with limitations according to avail- able man-hours
SAC operational apron, 118,153 sq yd of 17-in.-thick pavement	Rigid heavy	1959	FY 70: joints sealed on spalled areas in house	FY 72: repair pop-outs and spalls in house
ADC operational access apron 10,159 sq yd of 10-in.-thick pavement	Rigid light	1942 1955-56	FY 70-71: spalls repaired in house	Yearly in-house maintenance program
ADC operational apron, 88,676 sq yd of 10-in. and 10-7-10-in. pavement	Rigid light	1942 (small areas) 1955-56	FY 67: joints sealed FY 70: damaged slabs replaced (under contract) and spalls repaired in house	FY 72: repair spalls in house
ADC connecting taxiway 27,744 sq yd of 14- and 6-in.- thick pavement	Rigid light	1942	Yearly in-house maintenance program	FY 72: Project WUR 130-6 (completed Oct 71). Project WUR 135-8: repair power check pad (under contract)
SAC alert apron, 36,250 sq yd of 20-in.-thick pavement	Rigid heavy	1959	FY 67: joints sealed under con- tract. Yearly in-house main- tenance program	FY 72: repair pop-outs and spalls in house
Taxiway and apron stabilized shoulders	Flexible light	1959	FY 70-71: random cracks sealed in house	Yearly in-house maintenance program. Project WUR 86-6 being reviewed
Warm-up apron, 17,916 sq yd of 20-in.-thick pavement	Rigid heavy	1959	FY 67: joints sealed under contract FY 68: pop-outs repaired. Yearly in-house maintenance program	Yearly in-house maintenance program
ADC power check pad, 300 sq yd of 6-in.-thick pavement	Rigid light	1942	FY 63: pop-outs repaired and joints sealed under contract. Yearly in-house maintenance program	Yearly in-house maintenance program
Airfield drainage facilities	--	1942-60	FY 63: additional facilities constructed under contract	Yearly in-house maintenance program